

Tipos de elemento Ejercicio elemento cuatro nodos

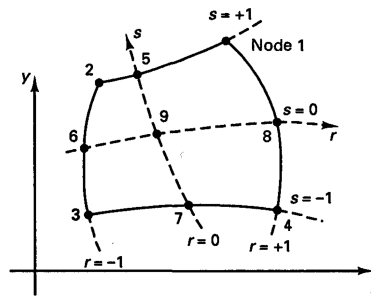
Dr. Alejo O. Sfriso
 Universidad de Buenos Aires
 SRK Consulting (Argentina)
 AOSA

materias.fi.uba.ar/6408
 latam.srk.com
 www.aosa.com.ar

asfriso@fi.uba.ar
 asfriso@srk.com.ar
 asfriso@aosa.com.ar

Elemento 2D de 4 a 9 nodos

Para aumentar la precisión se pueden agregar más elementos o usar elementos con mas nodos



(a) 4 to 9 variable-number-nodes two-dimensional element

Cuadrado

- 4 nodos: interpolación bi-lineal

| | Include only if node <i>i</i> is defined | | | | |
|---------------------------------|--|-------------------|-------------------|-------------------|-------------------|
| | <i>i</i> = 5 | <i>i</i> = 6 | <i>i</i> = 7 | <i>i</i> = 8 | <i>i</i> = 9 |
| $h_1 = \frac{1}{4}(1+r)(1+s)$ | $-\frac{1}{2}h_5$ | | | $-\frac{1}{2}h_8$ | $-\frac{1}{4}h_9$ |
| $h_2 = \frac{1}{4}(1-r)(1+s)$ | $-\frac{1}{2}h_5$ | $-\frac{1}{2}h_6$ | | | $-\frac{1}{4}h_9$ |
| $h_3 = \frac{1}{4}(1-r)(1-s)$ | | $-\frac{1}{2}h_6$ | $-\frac{1}{2}h_7$ | | $-\frac{1}{4}h_9$ |
| $h_4 = \frac{1}{4}(1+r)(1-s)$ | | | $-\frac{1}{2}h_7$ | $-\frac{1}{2}h_8$ | $-\frac{1}{4}h_9$ |
| $h_5 = \frac{1}{2}(1-r^2)(1+s)$ | | | | | $-\frac{1}{2}h_9$ |
| $h_6 = \frac{1}{2}(1-s^2)(1-r)$ | | | | | $-\frac{1}{2}h_9$ |
| $h_7 = \frac{1}{2}(1-r^2)(1-s)$ | | | | | $-\frac{1}{2}h_9$ |
| $h_8 = \frac{1}{2}(1-s^2)(1+r)$ | | | | | $-\frac{1}{2}h_9$ |
| $h_9 = (1-r^2)(1-s^2)$ | | | | | |

(Bathe 1996)

Ejemplo (mecánica continuo)

$$\underline{u}[x,y] = \underline{x}^{end} - \underline{x}^{ini} = \begin{Bmatrix} 1.5 + 0.1xy \\ 0.5 + 0.1xy \end{Bmatrix}$$

$$\underline{u}[0.5,0.4] = \begin{Bmatrix} 1.5 + 0.1x0.5x0.4 \\ 0.5 + 0.1x0.5x0.4 \end{Bmatrix} = \begin{Bmatrix} 1.52 \\ 0.52 \end{Bmatrix}$$

$$\underline{\underline{\epsilon}}[x,y] = \begin{Bmatrix} \frac{\partial u_1}{\partial x_1} & \frac{1}{2} \left(\frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right) \\ \frac{1}{2} \left(\frac{\partial u_1}{\partial x_2} + \frac{\partial u_2}{\partial x_1} \right) & \frac{\partial u_2}{\partial x_2} \end{Bmatrix} = \begin{Bmatrix} 0.1y & 0.05(x+y) \\ 0.05(x+y) & 0.1x \end{Bmatrix}$$

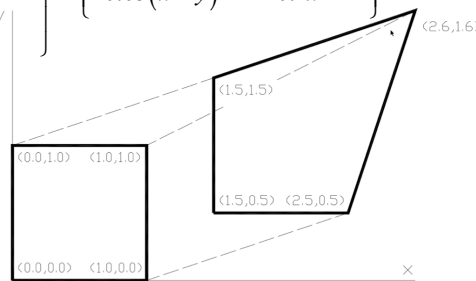
$$\underline{\underline{\epsilon}}[0.5,0.4] = \begin{Bmatrix} 0.04 & 0.045 \\ 0.045 & 0.05 \end{Bmatrix}$$

$$\frac{\partial u_1}{\partial x_1} = \frac{\partial [1.5 + 0.1xy]}{\partial x} = 0.1y$$

$$\frac{\partial u_1}{\partial x_2} = \frac{\partial [1.5 + 0.1xy]}{\partial y} = 0.1x$$

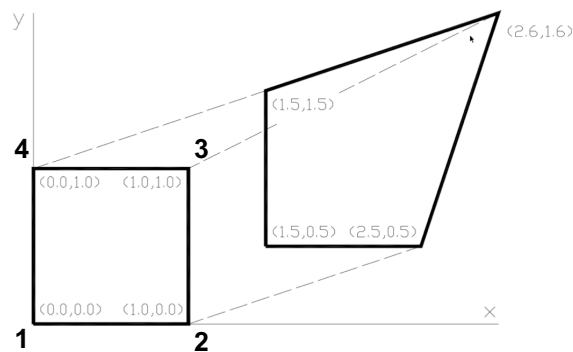
$$\frac{\partial u_2}{\partial x_1} = \frac{\partial [0.5 + 0.1xy]}{\partial x} = 0.1y$$

$$\frac{\partial u_2}{\partial x_2} = \frac{\partial [0.5 + 0.1xy]}{\partial y} = 0.1x$$



3

Ejemplo 2D: desplazamientos FEM

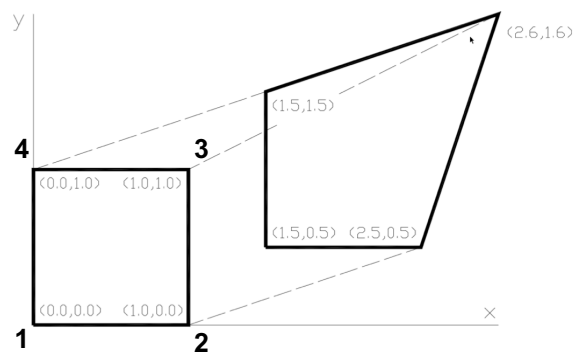


$$\underline{u}_e = \mathbf{f}[\underline{U}_e] = \mathbf{H} \times \underline{U}_e$$

$$\underline{u}_e = \mathbf{H} \cdot \underline{U}_e = \mathbf{H} \cdot \begin{Bmatrix} U_1 \\ V_1 \\ U_2 \\ V_2 \\ U_3 \\ V_3 \\ U_4 \\ V_4 \end{Bmatrix} = \mathbf{H} \cdot \begin{Bmatrix} 1.5 \\ 0.5 \\ 1.5 \\ 0.5 \\ 1.6 \\ 0.6 \\ 1.5 \\ 0.5 \end{Bmatrix}$$

4

Ejemplo 2D: desplazamientos FEM



$$\mathbf{H} = \begin{bmatrix} (1-x)(1-y) & 0 & x(1-y) & 0 & xy & 0 & (1-x)y & 0 \\ 0 & (1-x)(1-y) & 0 & x(1-y) & 0 & xy & 0 & (1-x)y \end{bmatrix}$$

Ejemplo 2D: desplazamientos FEM

$$\mathbf{H} = \begin{bmatrix} (1-x)(1-y) & 0 & x(1-y) & 0 & xy & 0 & (1-x)y & 0 \\ 0 & (1-x)(1-y) & 0 & x(1-y) & 0 & xy & 0 & (1-x)y \end{bmatrix}$$

$$u_e = \mathbf{H} \cdot \mathbf{U}_e = \begin{bmatrix} 0.3 & 0 & 0.3 & 0 & 0.2 & 0 & 0.2 & 0 \\ 0 & 0.3 & 0 & 0.3 & 0 & 0.2 & 0 & 0.2 \end{bmatrix} \begin{bmatrix} 1.5 \\ 0.5 \\ 1.5 \\ 0.5 \\ 1.6 \\ 0.6 \\ 1.5 \\ 0.5 \end{bmatrix} = \begin{bmatrix} 1.52 \\ 0.52 \end{bmatrix}$$

$\mathbf{u}_e = \mathbf{f}[\mathbf{U}_e] = \mathbf{H} \times \mathbf{U}_e$

Ejemplo 2D: deformaciones FEM



$$\mathbf{B} = \frac{\partial \mathbf{H}}{\partial \mathbf{x}} = \begin{bmatrix} -(1-y) & 0 & 1-y & 0 & y & 0 & -y & 0 \\ 0 & -(1-x) & 0 & -x & 0 & x & 0 & 1-x \\ -(1-x)/2 & -(1-y)/2 & -x/2 & (1-y)/2 & x/2 & y/2 & (1-x)/2 & -y/2 \end{bmatrix}$$

$$\boldsymbol{\varepsilon} = \mathbf{B} \cdot \mathbf{U}_e = \begin{bmatrix} -0.6 & 0 & 0.6 & 0 & 0.4 & 0 & -0.4 & 0 \\ 0 & -0.5 & 0 & -0.5 & 0 & 0.5 & 0 & 0.5 \\ -0.25 & -0.3 & -0.25 & 0.3 & 0.25 & 0.2 & 0.25 & -0.2 \end{bmatrix} \begin{Bmatrix} 1.5 \\ 0.5 \\ 1.5 \\ 0.5 \\ 1.6 \\ 0.6 \\ 1.5 \\ 0.5 \end{Bmatrix} = \begin{Bmatrix} 0.04 \\ 0.05 \\ 0.045 \end{Bmatrix}$$

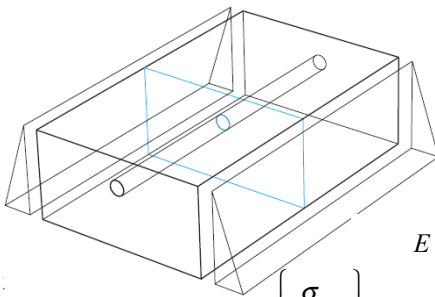
$$\mathbf{u}_e = \mathbf{H} \times \mathbf{U}_e$$

$$\boldsymbol{\varepsilon} = \mathbf{B} \times \mathbf{U}_e$$

Ejercicio elemento 4 nodos

7

Ejemplo 2D: tensiones (elasticidad)



$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & 1-\nu \end{bmatrix} \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{xy} \end{Bmatrix}$$

$$E = 1000 \text{ MPa} \quad \nu = 0.20$$

$$\begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} = \frac{1000 \text{ MPa}}{1-0.2^2} \begin{bmatrix} 1 & 0.2 & 0 \\ 0.2 & 1 & 0 \\ 0 & 0 & 0.8 \end{bmatrix} \begin{Bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \varepsilon_{xy} \end{Bmatrix}$$

$$\begin{Bmatrix} \mathbf{u}_e = \mathbf{f}[\mathbf{U}_e] \\ \boldsymbol{\varepsilon} = \mathbf{f}[\mathbf{u}_e] \\ \boldsymbol{\sigma} = \mathbf{f}[\boldsymbol{\varepsilon}] \end{Bmatrix} \begin{Bmatrix} \sigma_{xx} \\ \sigma_{yy} \\ \sigma_{xy} \end{Bmatrix} \Big|_{[0.5,0.4]} = \frac{1000 \text{ MPa}}{1-0.2^2} \begin{bmatrix} 1 & 0.2 & 0 \\ 0.2 & 1 & 0 \\ 0 & 0 & 0.8 \end{bmatrix} \begin{Bmatrix} 0.04 \\ 0.05 \\ 0.045 \end{Bmatrix} = \begin{Bmatrix} 52 \\ 60 \\ 38 \end{Bmatrix} \text{ MPa}$$

Ejercicio elemento 4 nodos

8